# An Evaluation of Slower Traffic Keep Right Signs and Pavement Markings 



Documents Prepared By:
Safety Evaluation Group
Traffic Safety Systems Management Unit Traffic Engineering and Safety Systems Branch
North Carolina Department of Transportation

Principal Investigator


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\frac{10 / 5 / 05}{\text { Date }}
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Traffic Safety Project Engineer


## INTRODUCTION

The desire to evaluate Slower Traffic Keep Right countermeasures came about due to increasing concerns regarding slower traffic using the left travel lanes and impeding faster moving traffic. North Carolina General Statute 20-146 makes it unlawful for any person to operate a motor vehicle in the inside lane of any dual-lane highway at a speed less than the posted speed limit when the vehicle impedes the steady flow of traffic and appropriate signs have been posted. "Appropriate signs" are defined as "Slower Traffic Keep Right" or designations of similar import. See Appendix A for complete text of General Statute 20-146.

The North Carolina Department of Transportation's current policy is to place "Slower Traffic Keep Right" signing based on engineering judgement. The signs are to be installed only on multi-lane freeways where motorists tend to drive in the left lane(s) at less than the posted speed limit and impede the flow of traffic. Signs are typically placed at three to five mile intervals to reinforce the behavior. The Department's complete policy can be found in Appendix B.

A work group consisting of senior level Traffic Engineering personnel has been charged with developing and evaluating measures to address this issue. Four countermeasures were identified by the group and evaluated under this project. The common goal of these countermeasures is to encourage slower drivers to stay out of the left lane by using various messages to influence their lane choice. The four countermeasures chosen for study were as follows:

- Keep Right Except to Pass Signs (See Figure 1)
- Keep Right Except to Pass Signs and Pass Lane Only Pavement Markings
- Slower Traffic Keep Right Signs (See Figure 2)
- Slower Traffic Keep Right Signs and Pass Lane Only Pavement Markings

Figure 1. Keep Right Except to Pass Sign


Figure 2. Slower Traffic Keep Right Sign

> SLOWER TRAFFIC KEEP RIGHT

## METHODOLOGY

## Site Selection

Sites were chosen for countermeasure installation based on the following criteria:

- Sites must be a four-lane divided freeway facility with minimum speed limit of 65 mph
- Sites must be away from interchanges or other features that would influence the lane choice of a driver
- Sites must be far away from each other so that the effect of one countermeasure does not interfere with the effect of another
- Sites must be far away from any other Slower Traffic Keep Right countermeasure

Figure 3 shows the location of each countermeasure installation along with the date of implementation. A map showing the countermeasure locations can be found in Appendix C of this report. Each countermeasure consisted of one sign or one sign and one pavement marking depending on the site. The countermeasures at Site 4 were installed in two phases and data was collected after each phase. See Appendix D for a detailed pavement marking and signing plan for sites where pavement markings were utilized.

Figure 3. Location of Countermeasures

| Site | Countermeasure | County | Route | Direction | Countermeasure <br> Installation Date |  |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: |
| 1 | Keep Right Except to Pass Signs | Pender | I-40 | WB | 0.60 miles East of SR 1411 Overpass (MM 405.8) | 4/29/2004 |
| 2 | Slow Traffic Keep Right Signs and Pass Lane Only Pavement Markings | Johnston | I-40 | WB | 0.60 miles East of SR 1322 Overpass (MM 320.7) | 9/14/2004 |
| 3 | Slow Traffic Keep Right Signs | Wake | I-40 | EB | 0.60 miles West of SR 2700 Overpass | $9.14 / 2004$ |
| 4 | Keep Right Except to Pass Signs and Pass Lane Only Pavement Markings | Wake | US 1 | SB | 0.60 miles North of SR 1149 | 3/22005 |
| 5 | Do Nothing Site - No Countermeasure Installed | Wake | US 1 | NB | At Railroad Bridge Near Chatham County Line | N/A |

Site 5 was chosen as a "Do Nothing Site" where no countermeasures were installed. Data at this site was gathered to attempt to account for any other factors that may have impacted the data collection (ex. accuracy drift in radar gun, system-wide changes in driver behavior). The Do Nothing site can not account for every factor, but was mainly included to show that the data would be relatively constant in the before and after periods if no countermeasure was installed.

## Data collection

All data collection was done in off peak hours, between the hours of 10:00 AM and 2:00 PM on typical weekdays, when it was likely that right lanes were accessible to slower drivers. Data collection was done in an inconspicuous manner so as not to influence the driver's speed or lane choice. Data was collected at each site for two hours on at least two different visits before and after the countermeasures were installed. This provided for four hours of data in the before period and four hours in the after period at each site.

The following measures of effectiveness (MOEs) were collected at each site.

- Speed distribution by lane - All speed data was captured using a radar gun. Speed data was collected to determine what effect the countermeasures would have on system speeds in the area of the signs
- Traffic volume by lane - Traffic volume was recorded to determine the effect the signs would have on lane utilization.
- Platooned vehicles in left lane - Data on the number of platooned vehicles was gathered to get some idea of how many vehicles were being impeded by slower traffic in the left lane.
- Lane changes just after a sign or pavement marking - Data on the number of lane changes in the vicinity of the sign was gathered to determine if drivers seemed to be making erratic maneuvers in the vicinity of the countermeasures.

The after period data was collected once the countermeasure had been in place for at least three weeks. This was done so that any novelty effect caused by the countermeasures would not impact the data.

## RESULTS

## Speed Analysis

Figures 4 through 6 below show the results of the speed analysis. More detailed speed data can be found in Appendix E of this report.

Figure 4. Left Lane Speed Analysis

|  |  |  |  | Left Lane |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CM Site | County | Location | Before / After | Cass | Trucks | \% Trucks | Volume | Speed Obs | $\begin{gathered} \text { Avg } \\ \text { Speed } \end{gathered}$ | 85th <br> Speed | Variance |
| 1 | Pender | I-40 | Before | 469 | 27 | 5\% | 496 | 290 | 75.1 | 78.5 | 20.8 |
| 1 | Pender | I-40 | After | 373 | 23 | 6\% | 396 | 170 | 75.8 | 78.8 | 19.2 |
| 2 | Johnston | I-40 | Before | 1225 | 96 | 7\% | 1321 | 491 | 75.4 | 79.1 | 19.2 |
| 2 | Johnston | I-40 | After | 1114 | 112 | 9\% | 1226 | 380 | 75.6 | 78.9 | 17.7 |
| 3 | Wake | I-40 | Before | 2744 | 135 | 5\% | 2879 | 700 | 74.3 | 77.9 | 16.8 |
| 3 | Wake | I-40 | After | 2473 | 103 | 4\% | 2576 | 443 | 74.9 | 78.1 | 12.8 |
| 4 | Wake | US 1 | Before | 344 | 46 | 12\% | 390 | 249 | 72.4 | 75.8 | 18.1 |
| 4 | Wake | US 1 | After Sign | 302 | 54 | 15\% | 356 | 177 | 72.6 | 76.0 | 18.7 |
| 4 | Wake | US 1 | After Sign \& Pvint Mkng | 305 | 44 | 13\% | 349 | 149 | 72.0 | 76.0 | 21.6 |
| 5 | Wake | US 1 | Before | 386 | 38 | 9\% | 424 | 284 | 72.2 | 76.2 | 24.2 |
| 5 | Wake | US 1 | After | 384 | 27 | 7\% | 411 | 227 | 72.2 | 75.7 | 17.9 |

Figure 5. Right Lane Speed Analysis

|  |  |  | Right Lane |  |  |  |  |  |  |  |  |
| :---: | :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CM Site | County | Location | Before/After | Cars | Trucks | \% Trucks | Volume | Speed <br> Obs | Avg <br> Speed | Sisth <br> Speed | Variance |
| 1 | Pender | I-40 | Before | 1287 | 260 | $17 \%$ | 1547 | 411 | 71.4 | 75.8 | 22.5 |
| 1 | Pender | I-40 | After | 1195 | 219 | $15 \%$ | 1414 | 454 | 72.0 | 75.6 | 20.6 |
| 2 | Johnston | I-40 | Before | 1828 | 537 | $23 \%$ | 2365 | 564 | 71.1 | 75.5 | 21.8 |
| 2 | Johnston | I-40 | After | 1869 | 566 | $23 \%$ | 2435 | 780 | 71.6 | 75.7 | 22.6 |
| 3 | Wake | I-40 | Before | 2916 | 673 | $19 \%$ | 3589 | 761 | 70.1 | 74.3 | 20.9 |
| 3 | Wake | I-40 | After | 2837 | 608 | $18 \%$ | 3445 | 774 | 70.6 | 74.8 | 21.8 |
| 4 | Wake | US 1 | Before | 1042 | 405 | $28 \%$ | 1447 | 395 | 68.7 | 72.5 | 17.2 |
| 4 | Wake | US 1 | After Sign | 1097 | 354 | $24 \%$ | 1451 | 504 | 68.2 | 72.3 | 20.7 |
| 4 | Wake | US 1 | After Sign \& Pvmt Mkng | 1162 | 362 | $24 \%$ | 1524 | 525 | 68.4 | 72.2 | 19.1 |
| 5 | Wake | US 1 | Before | 1086 | 314 | $22 \%$ | 1400 | 430 | 68.1 | 72.0 | 21.8 |
| 5 | Wake | US 1 | After | 1116 | 245 | $18 \%$ | 1361 | 481 | 68.3 | 72.2 | 20.9 |

Figure 6. Speed Analysis for Both Lanes Combined

|  |  |  |  |  | Both Lanes |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CM Site | County | Location | Before / After | \% Traffic Using Left Lane | Volume | Cass | Trucks | \% Trucks | Speed Obs | $A v g$ Speed | $\begin{gathered} \text { s5th } \\ \text { Speed } \\ \hline \end{gathered}$ | Variance |
| 1 | Pender | I-40 | Before | 24\% | 2043 | 1756 | 287 | 14\% | 701 | 72.9 | 77.1 | 25.1 |
| 1 | Pender | I-40 | After | 22\% | 1810 | 1568 | 242 | 13\% | 624 | 73.0 | 77.0 | 23.0 |
| 2 | Johnston | I-40 | Before | 36\% | 3686 | 3053 | 633 | 17\% | 1055 | 73.1 | 77.7 | 25.3 |
| 2 | Johnston | I-40 | After | 33\% | 3661 | 2983 | 678 | 19\% | 1160 | 72.9 | 77.3 | 24.5 |
| 3 | Wake | I-40 | Before | 45\% | 6468 | 5660 | 808 | 12\% | 1461 | 72.1 | 76.6 | 23.4 |
| 3 | Wake | I-40 | After | 43\% | 6021 | 5310 | 711 | 12\% | 1217 | 72.2 | 76.6 | 22.7 |
| 4 | Wake | US 1 | Before | 21\% | 1837 | 1386 | 451 | 25\% | 644 | 70.1 | 74.3 | 20.7 |
| 4 | Wake | US 1 | AfterSign | 20\% | 1807 | 1399 | 408 | 23\% | 681 | 69.3 | 73.8 | 23.8 |
| 4 | Wake | US 1 | After Sign \& Pvint Mkng | 19\% | 1873 | 1467 | 406 | 22\% | 674 | 69.2 | 73.4 | 21.8 |
| 5 | Wake | US 1 | Before | 23\% | 1824 | 1472 | 352 | 19\% | 714 | 69.7 | 73.8 | 26.7 |
| 5 | Wake | US 1 | After | 23\% | 1772 | 1500 | 272 | 15\% | 708 | 69.6 | 73.6 | 23.1 |

As can be seen in the table of results above, there was little change in average and $85^{\text {th }}$ percentile speeds between the two data collection periods. There was a statistically significant change in several of the speed categories when looking at each lane separately. However, the actual difference in speeds is very small and not practically significant. The numbers can be deemed statistically significant because of the large sample size of observations in each category. The large sample sizes allow for very small changes to be detected and declared significant in the statistical tests.

When looking at the speed data for both lanes together in Figure 6, there is a statistically significant change in the average speeds for Site 4 and a statistically significant change in the $85^{\text {th }}$ percentile speeds at Sites 2 and 4. Again the actual numerical difference in speeds is quite small and not practically significant. Further investigation shows that the statistically significant changes are actually cases where the speeds have dropped from the before to the after period.

The data in Figures 4 through 6 show actual raw data. Ordinarily, the data is adjusted to incorporate the change seen at the Do Nothing Site in an attempt to account for any changes that may have occurred between the before and after periods not related to the applied countermeasure. In this case, there was so little change from the before to the after period at the

Do Nothing Site that it makes no practical difference in the outcome of this study. For completeness, the adjustment exercise was carried out and can be found in Appendix F.

## Analysis of Traffic Using Left Lane

Figure 6 shows the before and after values for the percentage of vehicles using the left lane. At each site except for the Do Nothing Site, there was about a $2-3$ percent reduction in the number of vehicles traveling in the left lane after the countermeasure was installed. Comparing the change in the percent of traffic in the left lane by itself can be very misleading. The largest factor that plays a role in the amount of traffic in the left lane is the volume of traffic using the facility. As the overall traffic volumes increase, the number of vehicles in the left lane will increase in order to handle the additional traffic. One can imagine as a facility approaches capacity, left lane usage will approach $+/-50$ percent simply as a way to move the most amount of traffic. Taking overall volume fluctuations into consideration, it appears Sites 2 and 4 show the most evidence of a possible reduction of vehicles in the left lane. In each of these sites, the volume remained relatively the same in the before and after periods and the percent of traffic in the left lane went down slightly.

## Analysis of Platooned Vehicles

Figure 7 below shows the results of the analysis of platooned vehicles in left lane.
Figure 7. Platooned Vehicle Data

| CM Site | Before / After | Number of <br> Platooned Vehicles | Number of Vehicles <br> in Left Lane | Percent of Left Lane <br> Vehicles In Platoon | Number of Observed <br> Lane Changes |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | Before | 77 | 496 | $16 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 1 | After | 45 | 396 | $11 \%$ | 40 |
| 2 | Before | 368 | 1321 | $28 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 2 | After | 360 | 1226 | $29 \%$ | 93 |
| 3 | Before | 960 | 2879 | $33 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 3 | After | 1001 | 2576 | $39 \%$ | 46 |
| 4 | Before | 51 | 390 | $13 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 4 | After Sign | 47 | 356 | $13 \%$ | 39 |
| 4 | After Sign \& Pvmt Mkng | 52 | 349 | $15 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 5 | Before | 53 | 424 | $13 \%$ | $\mathrm{~N} / \mathrm{A}$ |
| 5 | After | 48 | 411 | $12 \%$ | 36 |

The only sites that showed a decrease in the percentage of vehicles in the left lane that were in a platoon were Sites 1 and the Do Nothing Site. The other sites all showed a slight increase in the percentage of platooned vehicles. Because of the low sample sizes, none of these changes can be deemed statistically significant except for the change at Site 3 which was an increase.

Observations regarding the number of lane changes in the vicinity of the countermeasures were also recorded to ensure drivers were not making erratic movements after seeing the signs or pavement markings. There was some concern that drivers may immediately try to change lanes
after passing the countermeasure and attempt an unsafe merge. The project team did not note any drivers behaving abnormally after passing by the countermeasures.

## FINAL COMMENTS

The before period data at each of the sites show that most drivers tend to travel in the right lane as a general rule even without a countermeasure in place. Drivers seem to have been trained into behaving this way either by experience, driver education classes (Department of Motor Vehicles Handbook discusses this issue), or by noting the various "Slower Traffic Keep Right" signs that are scattered about multi-lane roads in North Carolina. It seems the greatest factor that impacts left lane usage is the volume on the facility. As volume increases, traffic must adjust and utilize the left lane more in order to accommodate the additional vehicles.

None of the countermeasures analyzed seemed to have a significant impact on the measures of effectiveness chosen for this study. Sites where pavement markings and signs were placed together (Sites 2 and 4) seemed to be the most effective at reducing the amount of traffic in the left lane, but the changes are still quite small and the benefits seem negligible when considering the other performance measures. Based on the results of this study, it is unlikely to see significant benefits at a particular site by implementing the countermeasures considered for this project. However, placing signs at appropriate sites and intervals may help to reinforce the notion of slower traffic keep right on a system-wide basis. This practice is in concurrence with the current policy used by the Department.

The issue of getting slower traffic to stay in the right most lanes when available is deeply rooted in the realm of human factors research. The purpose of this project was not to understand why slower drivers sometimes travel in the left lane, but to determine if a set of predetermined, readily available countermeasures have any significant effect in urging slower drivers to move out of the left lane. Human factors research is playing an increasingly important role in transportation issues today and how best to encourage slower drivers to move to the right seems to be an excellent candidate for this type of research. Understanding why some slower drivers tend to drive in the left lane when the right lane is available may lead to better countermeasures that are more effective in urging drivers to move over.
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Please direct questions or comments on this evaluation to Brian Murphy, PE at (919) 733-3915 or via email at bgmurphy@dot.state.nc.us

## APPENDIX A

## § 20-146. Drive on right side of highway; exceptions.

(a) Upon all highways of sufficient width a vehicle shall be driven upon the right half of the highway except as follows:
(1) When overtaking and passing another vehicle proceeding in the same direction under the rules governing such movement;
(2) When an obstruction exists making it necessary to drive to the left of the center of the highway; provided, any person so doing shall yield the right-of-way to all vehicles traveling in the proper direction upon the unobstructed portion of the highway within such distance as to constitute an immediate hazard;
(3) Upon a highway divided into three marked lanes for traffic under the rules applicable thereon; or
(4) Upon a highway designated and signposted for one-way traffic.
(b) Upon all highways any vehicle proceeding at less than the legal maximum speed limit shall be driven in the right-hand lane then available for thru traffic, or as close as practicable to the right-hand curb or edge of the highway, except when overtaking and passing another vehicle proceeding in the same direction or when preparing for a left turn.
(c) Upon any highway having four or more lanes for moving traffic and providing for two-way movement of traffic, no vehicle shall be driven to the left of the centerline of the highway, except when authorized by official traffic-control devices designating certain lanes to the left side of the center of the highway for use by traffic not otherwise permitted to use such lanes or except as permitted under subsection (a)(2) hereof.
(d) Whenever any street has been divided into two or more clearly marked lanes for traffic, the following rules in addition to all others consistent herewith shall apply.
(1) A vehicle shall be driven as nearly as practicable entirely within a single lane and shall not be moved from such lane until the driver has first ascertained that such movement can be made with safety.
(2) Upon a street which is divided into three or more lanes and provides for the two-way movement of traffic, a vehicle shall not be driven in the center lane except when overtaking and passing another vehicle traveling in the same direction when such center lane is clear of traffic within a safe distance, or in the preparation for making a left turn or where such center lane is at the time allocated exclusively to traffic moving in the same direction that the vehicle is proceeding and such allocation is designated by official traffic-control device.
(3) Official traffic-control devices may be erected directing specified traffic to use a designated lane or designating those lanes to be used by traffic moving in a particular direction regardless of the center of the street and drivers of vehicles shall obey the direction of every such device.
(4) Official traffic-control devices may be installed prohibiting the changing of lanes on sections of streets, and drivers of vehicles shall obey the directions of every such device.
(e) Notwithstanding any other provisions of this section, when appropriate signs have been posted, it shall be unlawful for any person to operate a motor vehicle over and upon the inside lane, next to the median of any dual-lane highway at a speed less than the posted speed limit when the operation of said motor vehicle over and upon said inside lane shall impede the steady flow of traffic except when preparing for a left turn. "Appropriate signs" as used herein shall be construed as including "Slower Traffic Keep Right" or designations of similar import. (1937, c. 407, s. 108; 1965, c. 678, s. 2; 1973, c. 1330 , s. 3 ; 1975, c. 593 ; 1985, c. 764 , s. 25 ; 1985 (Reg. Sess., 1986), c. 852, s. 17 ; 2001-487, s. 11.)

APPENDIX B

# State of North Carolina <br> DEPARTMENT OF TRANSPORTATION 

Michael F. Easley
GOVERNOR

Lyndo Tippett
Secretary

October 21, 2003

MEMORANDUM
To:
From:
W. S. Varnedoe, P.E.


Subject: Slower Traffic Keep Right Signing
Attached are guidelines on the "Standard Practice for Slower Traffic Keep Right Signing". These guidelines are intended to ensure statewide consistency in the installation and maintenance of appropriate signs on the highway right-of-way identifying locations to the general public where Slower Traffic Keep Right is applicable.

These guidelines take effect immediately. If further information is needed, please contact either of us.

WSV:TAP:la
Attachment
cc: Len A. Sanderson, P.E., w/att.
Regional Traffic Engineers, w/att.
Division Traffic Engineers, w/att.
Sign. Practices Technical Committee, w/att.
Sign Oversight Committee, w/att.
Sandy Nance, w/att.
Ken Ivey, P.E., w/att.

# North Carolina Department of Transportation Division of Highways <br> Traffic Engineering and Safety Systems Branch 

STANDARD PRACTICE
for
Slower Traffic Keep Right Signing
It is the standard practice of NCDOT, based on G.S. 20-146(e), to install "Slower Traffic Keep Right" (R4-3) regulatory signs, based on engineering judgement. The signs shall be located and erected according to the standards of the Manual on Uniform Traffic Control Devices(MUTCD), the North Carolina Supplement to the MUTCD, and the North Carolina Roadway Standard Drawings.

## CRITERIA

Signs are restricted to multi-lane facilities with full control of access at locations where motorists tend to drive in the left lane(s) at less than the posted speed limit and impede the flow of traffic.

- Signs are not to be used on an approach to an interchange, through an interchange area, or in high volume urban areas where there is a capacity deficiency.
- Signs are not to be used on roadway facilities if there are other lane restrictions in place, on roadways under construction, or on facilities with poor roadway pavement conditions that affects the ride-ability of the travel lanes.
- Signs are to be installed on the median side of the highway and behind existing guardrail, when possible.

Signs are to be placed at three (3) to five (5) mile intervals.

- Signs shall conform to the attached NCDOT approved Standards

Note: For "Slower Traffic Keep Right" signing, pavement marking and other countermeasures see TEPPL Topic K-1 (Keep Right Except to Pass)

## G.S. 20-146. Drive on right side of highway; exceptions. (e)

Not withstanding any other provisions of this section, when appropriate signs have been posted, it shall be unlawful for any person to operate a motor vehicle over and upon the inside lane, next to the median of any dual-lane highway at a speed less than the posted speed limit when the operation of said motor vehicle over and upon said inside lane shall impede the steady flow of traffic except when preparing for a left turn. "Appropriate signs" as used herein shall be construed as including "Slower Traffic Keep Right" or designations of similar import.




APPENDIX C

## COUNTERMEASURE LOCATIONS



## APPENDIX D



## APPENDIX E

Detailed Speed Data


## APPENDIX F

Speed Data Adjusted By Do Nothing Site Data

|  |  |  |  | Leff Lane |  |  |  |  |  |  |  | Right Lane |  |  |  |  |  |  |  |  | Both Lanes |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| cı Site | County | Location | Before / After | Cars | Trucks | \% Trucks | Volume | $\left\lvert\, \begin{gathered} \text { Speed } \\ \text { obs } \end{gathered}\right.$ | $\begin{array}{\|l\|l} \text { Avg } \\ \text { Speed } \end{array}$ | $\begin{array}{\|l\|l} \text { 8sth } \\ \text { Sppeed } \end{array}$ | Variance | Cars | Trucks | \% Trucks | Volume | $\begin{array}{\|l\|l} \substack{\text { Soeed } \\ \hline \text { bs }} \end{array}$ | $\begin{aligned} & \text { Spg } \\ & \text { Speed } \end{aligned}$ | $\begin{array}{\|l\|l\|} \text { 8sith } \\ \text { Sppeed } \end{array}$ | Variance | $\begin{gathered} \text { \% Traffic Using } \\ \text { Left Lane } \end{gathered}$ | Volume | Cars | Trucks | \% Trucks | $\left\|\begin{array}{c} \text { Speed } \\ \text { obs } \end{array}\right\|$ | $\begin{aligned} & \text { Avg } \\ & \text { Speed } \end{aligned}$ | $\left.\begin{aligned} & \text { sish } \\ & \text { Speed } \end{aligned} \right\rvert\,$ | Variance |
| 1 | Pender | I.40 | Before | 469 | 27 | 5\% | 496 | 290 | 75.1 | 78.5 | 20.8 | ${ }^{1287}$ | 260 | 17\% | 1547 | 411 | 71.4 | 75.8 | 22.5 | 24\% | 2043 | 1756 | 287 | 14\% | 701 | 72.9 | 77.1 | 25.1 |
| 1 | Pender | ${ }^{1} 40$ | Affer | 373 | 23 | 6\% | 396 | 170 | 75.7 | 78.3 | 19.2 | 1195 | 219 | 15\% | 1414 | 454 | 72.2 | 75.9 | 20.6 | 22\% | 1810 | 1568 | 242 | 13\% | 624 | 72.9 | 76.8 | 23.0 |
| 2 | Johnston | ${ }^{1} 40$ | Before | ${ }^{1225}$ | 96 | 7\% | 1321 | 491 | 75.4 | 79.1 | 19.2 | 1828 | 537 | 23\% | 2365 | 564 | ${ }_{7} 7.1$ | 75.5 | 21.8 | 36\% | 3686 | 3053 | 633 | 17\% | 1055 | 73.1 | 77.7 | 25.3 |
| 2 | Johnston | ${ }^{1} 40$ | After | 1114 | 112 | 9\% | 1226 | 380 | 75.6 | 78.3 | 17.7 | 1869 | 566 | 23\% | 2435 | 780 | ${ }^{71.8}$ | 76.0 | 22.6 | 33\% | 3661 | 2983 | 678 | 19\% | 1160 | 72.7 | 77.1 | 24.5 |
| 3 | Wake | ${ }^{1} 40$ | Before | 2744 | 135 | 5\% | 2879 | 700 | 74.3 | 77.9 | 16.8 | 2916 | 673 | 19\% | 3589 | 761 | 70.1 | 74.3 | 20.9 | 45\% | 6468 | 5660 | 808 | 12\% | 1461 | 72.1 | 76.6 | 23.4 |
| 3 | Wake | ${ }^{1} 40$ | After | 2473 | 103 | 4\% | 2576 | 443 | 74.9 | 77.6 | 12.8 | 2837 | 608 | 18\% | 3445 | 774 | 70.8 | 75.0 | 21.8 | 43\% | 6021 | 5310 | 711 | 12\% | 1217 | 72.0 | 76.4 | 22.7 |
| 4 | Wake | US 1 | Before | ${ }^{344}$ | 46 | 12\% | 390 | 249 | 72.4 | 75.8 | 18.1 | 1042 | 405 | 28\% | 1447 | 395 | 68.7 | 72.5 | 17.2 | 21\% | 1837 | 1386 | 451 | 25\% | 644 | 70.1 | ${ }^{74.3}$ | 20.7 |
| 4 | Wake | US 1 | After Sign | 302 | 54 | 15\% | 356 | 177 | 72.6 | 75.4 | 18.7 | 1097 | 354 | 24\% | 1451 | 504 | 68.4 | ${ }^{2} 2.6$ | 20.7 | 20\% | 1807 | 1399 | 408 | 23\% | 681 | 69.2 | ${ }^{73.6}$ | 23.8 |
| 4 | Wake | US 1 | After Sign \& Pvmt Mkng | 305 | 44 | 13\% | 349 | 149 | 72.0 | 75.4 | 21.6 | 1162 | 362 | 24\% | 1524 | 525 | 68.6 | 72.4 | 19.1 | 19\% | 1873 | 1467 | 406 | 22\% | 674 | 69.1 | 73.2 | 21.8 |
| 5 | Wake | US 1 | Before | 386 | 38 | 9\% | 424 | 284 | 72.2 | 76.2 | 24.2 | 1086 | 314 | 22\% | 1400 | 430 | 68.1 | 72.0 | 21.8 | 23\% | 1824 | 1472 | 352 | 19\% | 714 | 69.7 | 73.8 | 26.7 |
| 5 | Wake | US 1 | After | 384 | 27 | 7\% | 411 | 227 | 72.2 | 75.7 | 17.9 | 1116 | 245 | 18\% | 1361 | 481 | 68.3 | 72.2 | 20.9 | 23\% | 1772 | 1500 | 272 | 15\% | 708 | 69.6 | 73.6 | 23.1 |
| Adjustment Factors |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Leff Lane Average Speed |  |  | 1.00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Leff Lane 85th Percentile SpeedLeft Lane Average Speed |  |  | 0.99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 1.00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Leff Lane 85th Percentile SpeedBoth Lanes A verage speed |  |  | 1.00 1.00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | 1.00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

